

# Chapter 4: Axial Load

## Chapter Objectives

- ✓ Determine the elastic deformation of axially loaded members
- ✓ Apply the principle of superposition for total effect of different loading cases
- ✓ Deal with compatibility conditions
- ✓ Deal with thermal stresses
- ✓ Misfit problems



# Axial deformation

So far...

$$\sigma = \frac{F}{A} \quad \epsilon = \frac{\delta}{L} \quad \sigma = E \epsilon$$

$$\frac{F}{A} = E \frac{\delta}{L} \Rightarrow \delta = \frac{F \cdot L}{E \cdot A}$$

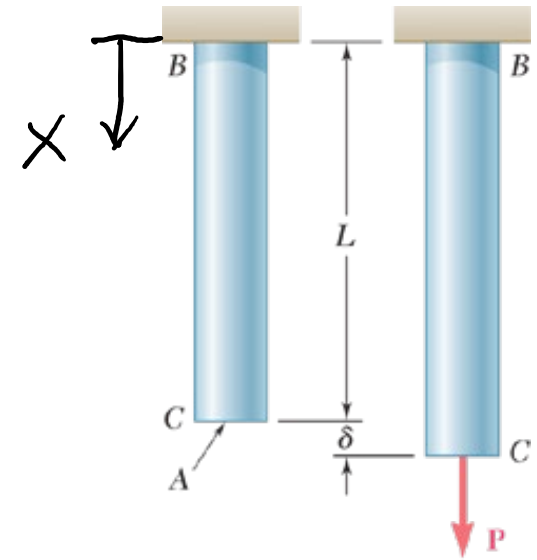
$$F = k \cdot \Delta x \Rightarrow F = k \cdot \delta = \left( \frac{EA}{L} \right) \cdot \delta$$

Spring const.

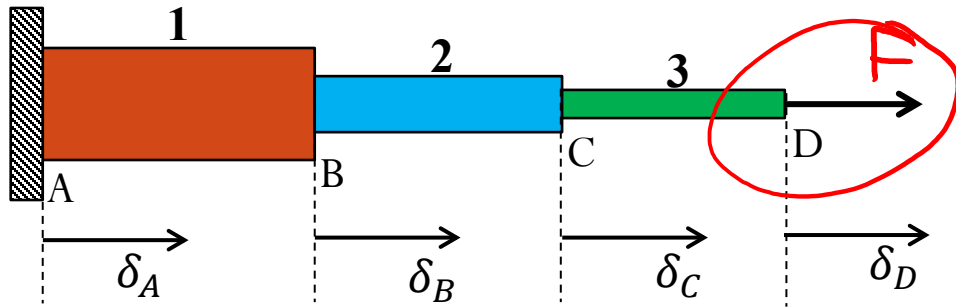
In general,  $\epsilon = \frac{du}{dx}$  ← local material displacement

$$\frac{F}{A} = E \cdot \frac{du}{dx} \Rightarrow du = \frac{F(x)}{E(x) A(x)} \cdot dx$$

$$U(x_2) - U(x_1) = \int_{x_1}^{x_2} \frac{F(x)}{E(x) A(x)} dx$$



Superposition principle: 1) The loading must be linearly related to the stress or displacement that is to be determined; 2) Small deformation assumption



$$\delta = \sum_i \delta_i = \sum_i \frac{F_i L_i}{E_i A_i}$$

$\delta_A = 0$  (A is fixed at wall)

$\delta_{B/A} = \delta_B - \delta_A$  (Relative displacement of two points)

$$\delta_1 = \delta_B - \delta_A = \frac{F_1 L_1}{E_1 A_1}$$

$$\delta_{C/B} = \delta_C - \delta_B = \delta_2 = \frac{F_2 L_2}{E_2 A_2}$$

$$\delta_{D/C} = \delta_D - \delta_C = \delta_3 = \frac{F_3 L_3}{E_3 A_3}$$

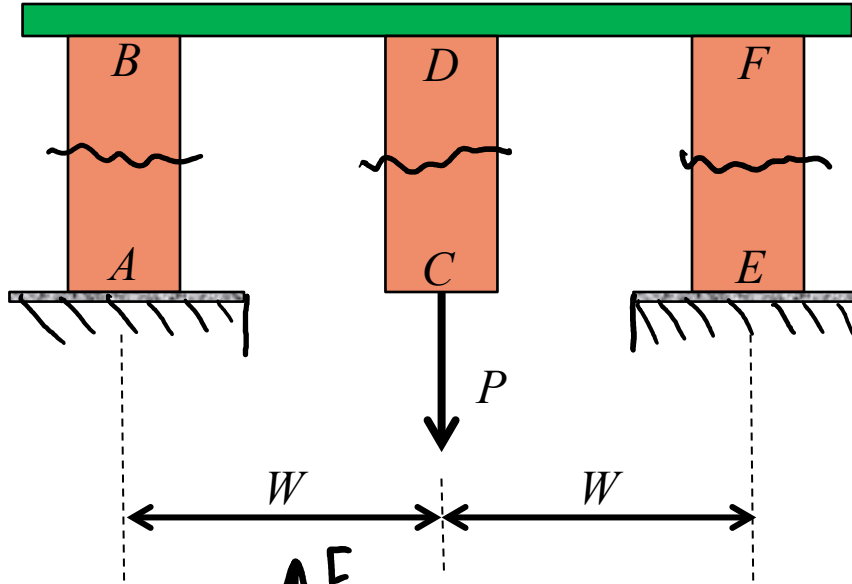
positive, so all rods increase in length

$F_3 = F$   
 $F_2 = F$   
 $F_1 = F$

A small diagram shows a rectangular rod segment with a force F3 acting to the left at the top and a force F acting to the right at the bottom.



# Example 1



Rigid bar

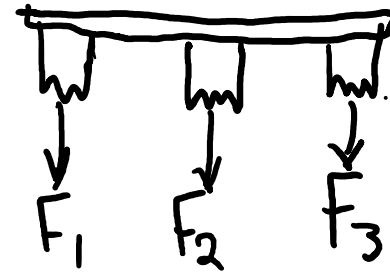
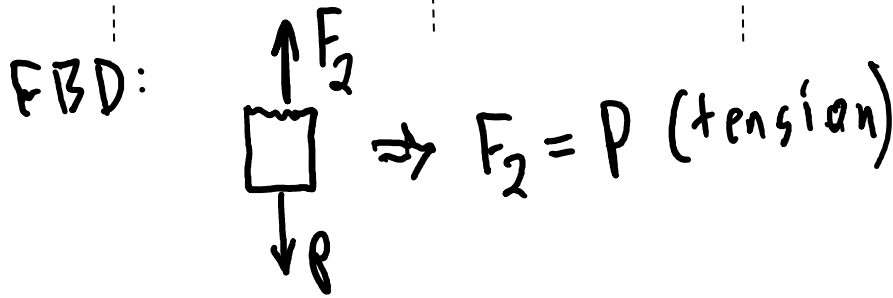
does not bend

Obtain the displacement of points C and D when the load P is applied

Ground is fixed

Assume each bar has length  $L$ ,  $E$ ,  $A$  all same

Symmetric.



$F_1 = F_3$   
(symmetry)

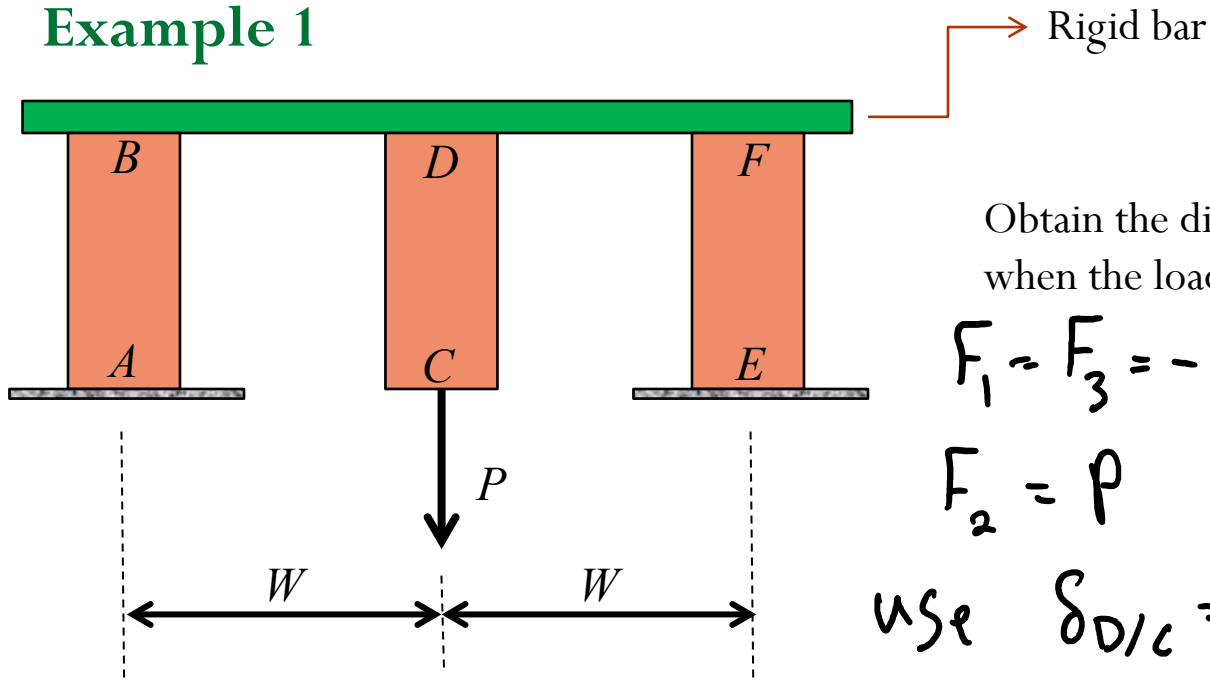
$$\sum F_y = 0 \Rightarrow -F_1 - F_2 - F_3 = 0$$

$$F_1 = F_3 = -\frac{1}{2}F_2 = -\frac{1}{2}P$$

(compression)



# Example 1



Obtain the displacement of points C and D when the load P is applied

$$F_1 = F_3 = -\frac{1}{2}P \quad \text{want } \delta_C \text{ and } \delta_D$$

$$F_2 = P$$

$$\text{Use } \delta_{D/C} = \delta_D - \delta_C = \delta_2 = \frac{F_2 L}{EA} = \frac{P \cdot L}{EA}$$

$$\delta_D = \delta_B = \delta_F \quad \text{because } \delta_A = \delta_E = 0$$

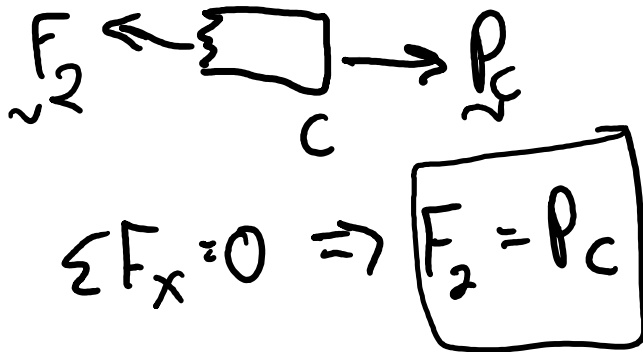
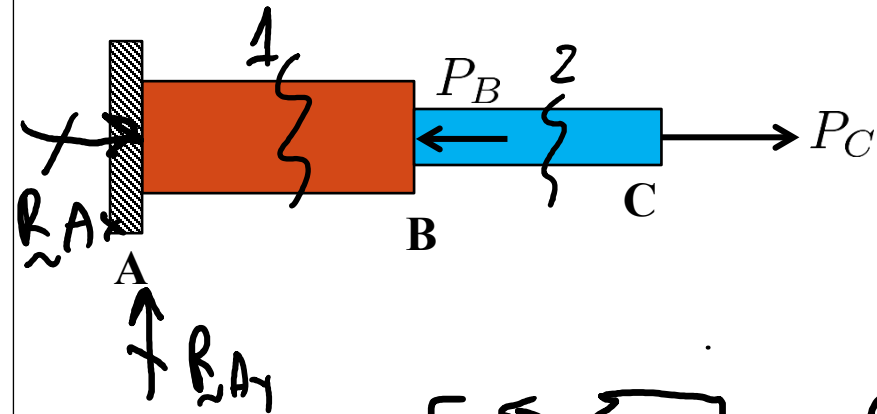
$$\delta_{B/A} = \delta_B - \delta_A = \frac{F_1 L}{EA} = -\frac{PL}{2EA} \Rightarrow \delta_B = \delta_D = \frac{-PL}{2EA}$$

$$\delta_C = \delta_D - \delta_{A/C} = \delta_D - \frac{F_2 L}{EA} = \frac{-PL}{2EA} - \frac{2PL}{2EA} = \frac{-3PL}{2EA}$$



# Statically Determinate Problems

can solve for all internal forces and reactions by using equilibrium alone.



$$\sum F_x = 0 \Rightarrow F_2 = P_C$$



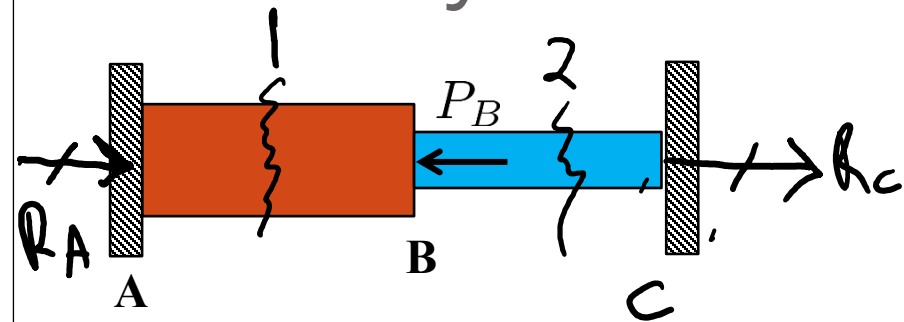
$$R_A = P_B - P_C$$

$$\sum F_x = 0 \Rightarrow -F_1 - P_B + P_C = 0$$

$$F_1 = P_C - P_B$$



# Statically Indeterminate Problems



Motion is constrained at both A & C  
 $\Rightarrow$  Two reactions

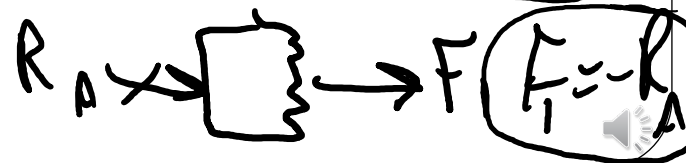
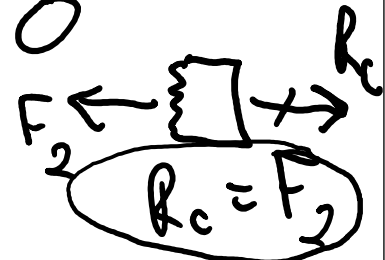
$\sum F_x = 0$  is the only equil. eqn. we can use

$$R_A - P_B + R_C = 0$$

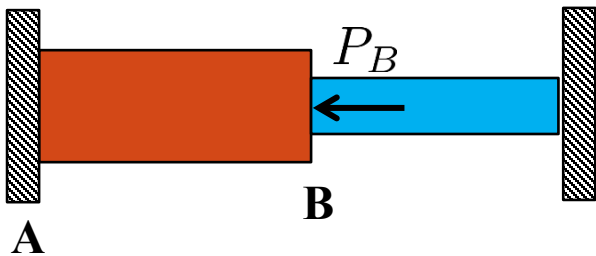
Apply compatibility condition.  $\Rightarrow$  Geometric constraint.  
 If A & C are fixed, then  $\delta_1 + \delta_2 = 0$

$$\frac{F_1 L_1}{E_1 A_1} + \frac{F_2 L_2}{E_2 A_2} = 0$$

$$|F_1| = |R_A| \quad |F_2| = |R_C|$$



# Statically Indeterminate Problems



$$\delta_1 + \delta_2 = 0$$

$$F_2 = F_1 + P_B$$

$$\frac{F_1 L_1}{E_1 A_1} + \frac{(F_1 + P_B) \cdot L_2}{E_2 A_2} = 0 \rightarrow \frac{F_1 L_1}{E_1 A_1} + \frac{F_1 L_2}{E_2 A_2} = \frac{-P_B L_2}{E_2 A_2}$$

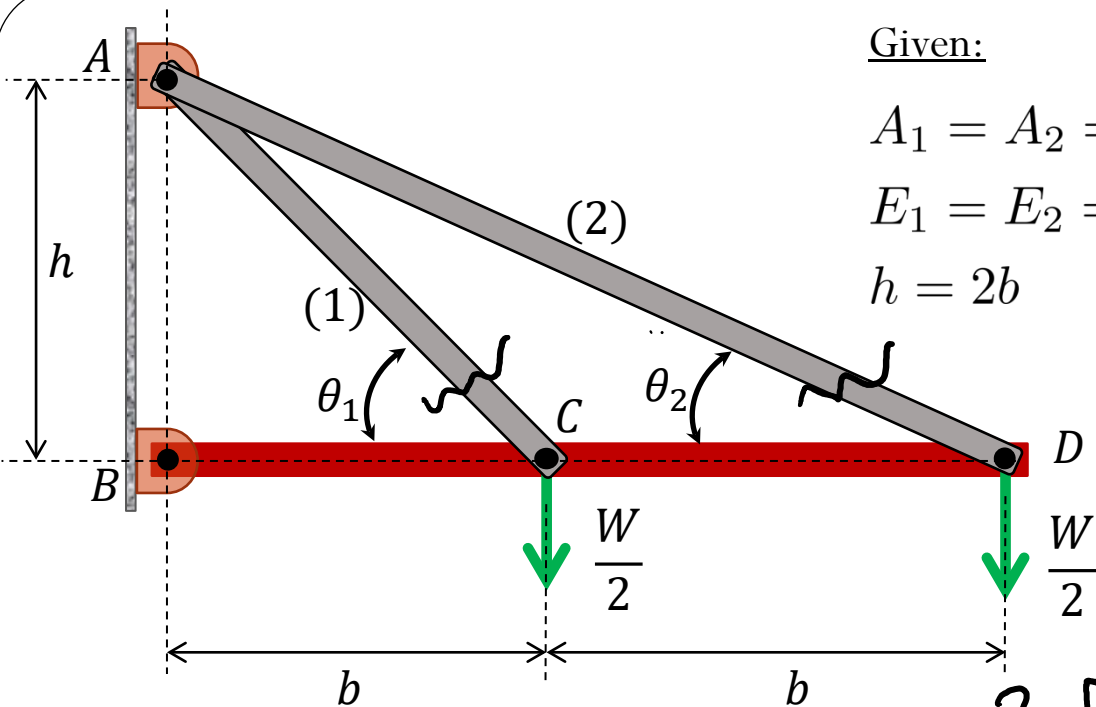
$$\rightarrow F_1 \left( \frac{L_1}{E_1 A_1} + \frac{L_2}{E_2 A_2} \right) = \frac{-P_B L_2}{E_2 A_2}$$

$$\rightarrow F_1 = \frac{-P_B L_2}{E_2 A_2 \left( \frac{L_1}{E_1 A_1} + \frac{L_2}{E_2 A_2} \right)}$$

... can also solve for  $F_2$







Given:

$$A_1 = A_2 = A$$

$$E_1 = E_2 = E$$

$$h = 2b$$

Find:

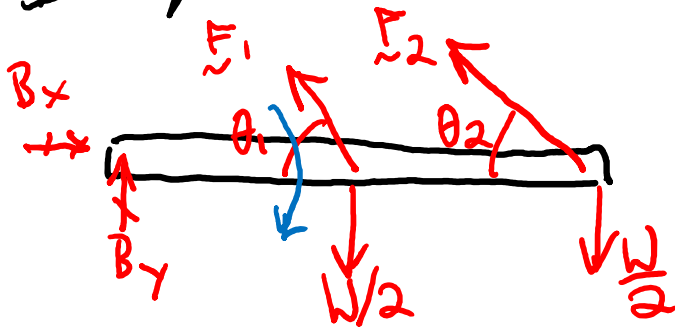
$$\sigma_1 = ?$$

$$\sigma_2 = ?$$

$$\delta_D = ?$$

Assume that BCD is rigid.

1. Equil: brium



$$\sum M_B = 0$$

$$-\frac{W}{2} \cdot b - Wb + F_1 \cdot \sin \theta_1 \cdot b + F_2 \sin \theta_2 \cdot 2b = 0$$

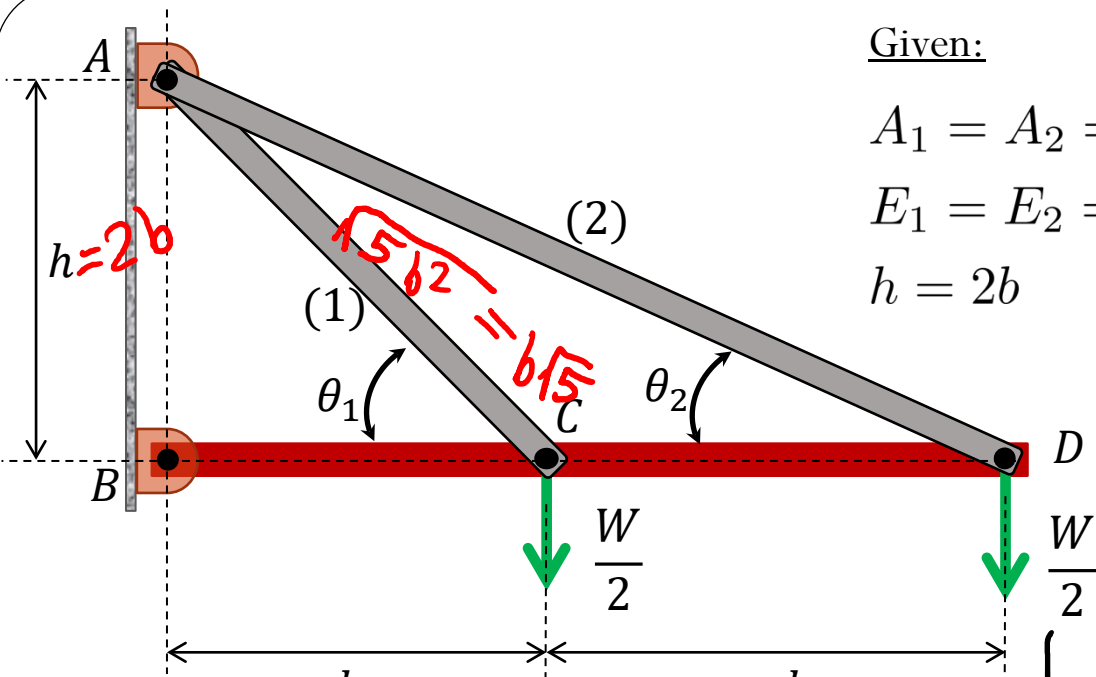
$$(1) F_1 \cdot \sin \theta_1 + 2 \cdot F_2 \cdot \sin \theta_2 = \frac{3}{2} W$$

2. Force-Elongation

$$(2) e_1 = \frac{F_1 \cdot L_1}{EA} \quad [e_i] = \text{length}$$

$$(3) e_2 = \frac{F_2 \cdot L_2}{EA}$$





Given:

$$A_1 = A_2 = A$$

$$E_1 = E_2 = E$$

$$h = 2b$$

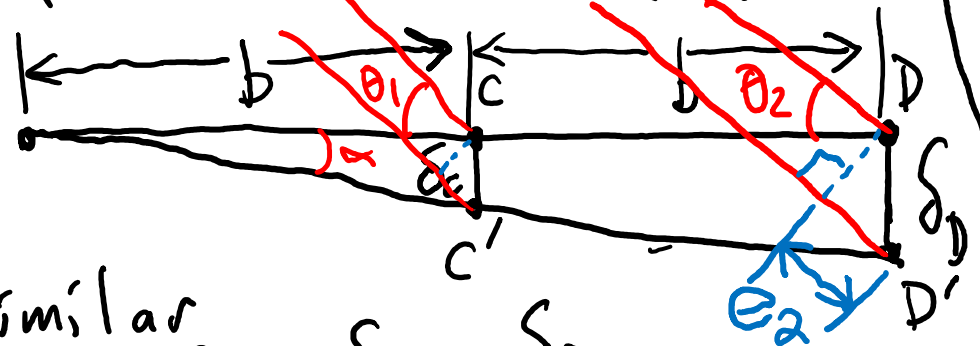
Find:

$$\sigma_1 = ?$$

$$\sigma_2 = ?$$

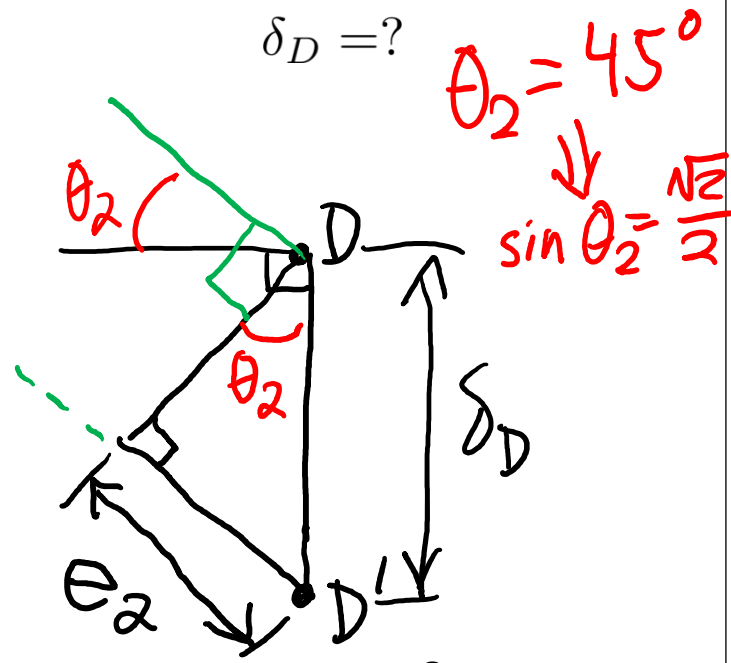
$$\delta_D = ?$$

### 3. Geometric Compatibility (assume small rotation)



similar triangles

$$\frac{\delta_c}{b} = \frac{\delta_D}{2b} \Rightarrow \delta_D = 2 \cdot \delta_c \quad (4)$$

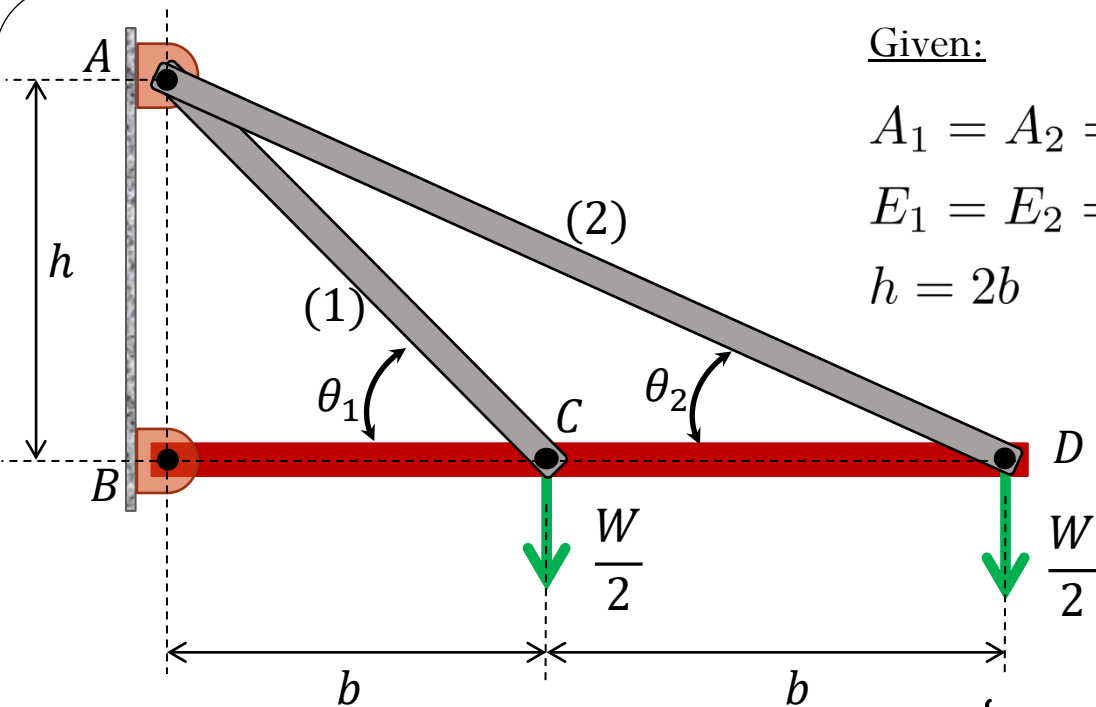


$$\sin \theta_2 = \frac{\delta_D}{\delta_c} \quad (5)$$

$$\sin \theta_1 = \frac{\delta_c}{b} \quad (6)$$

$$\sin \theta_1 = \frac{2}{\sqrt{5}}$$





Given:

$$A_1 = A_2 = A$$

$$E_1 = E_2 = E$$

$$h = 2b$$

Find:

$$\sigma_1 = ?$$

$$\sigma_2 = ?$$

$$\delta_D = ?$$

Apply  $\delta_D = 2\delta_C$

~~$$\frac{2F_2 b}{EA} = 2 \left( \frac{5F_1 b}{2EA} \right)$$~~

$$F_1 = \frac{2}{5} F_2$$

(3) into (5)

$$\frac{F_2 \cdot (b\sqrt{2})}{EA \cdot \delta_D} = \frac{\sqrt{2}}{2} \rightarrow \delta_D = \frac{2F_2 b}{EA}$$

(2) into (6)

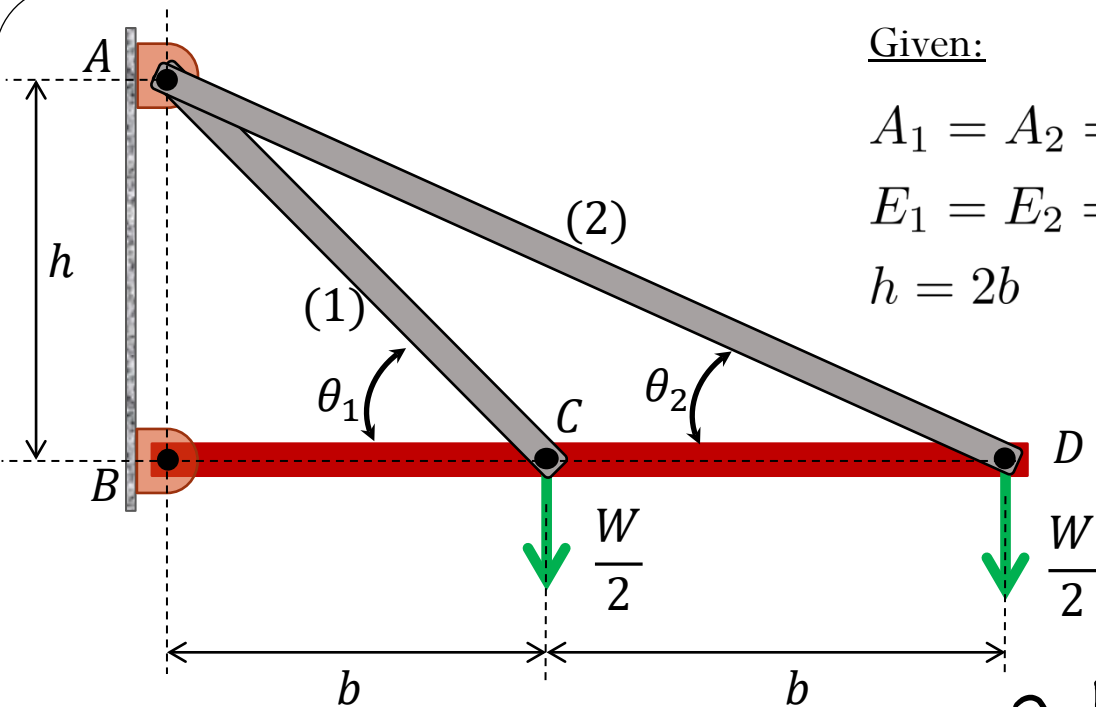
$$\frac{F_1 \cdot b\sqrt{5}}{EA \cdot \delta_C} = \frac{2}{\sqrt{5}} \rightarrow \delta_C = \frac{5F_1 b}{2EA}$$

Plug into (1) and solve

$$\frac{2}{5} F_2 \cdot \frac{2}{\sqrt{5}} + 2F_2 \frac{\sqrt{2}}{2} = \frac{3W}{2}$$

$$F_2 = \frac{75W}{8\sqrt{5} + 50\sqrt{2}}$$

$$F_1 = \frac{15W}{4\sqrt{5} + 25\sqrt{2}}$$



Given:

$$A_1 = A_2 = A$$

$$E_1 = E_2 = E$$

$$h = 2b$$

Find:

$$\sigma_1 = ?$$

$$\sigma_2 = ?$$

$$\delta_D = ?$$

$$\sigma_1 = \frac{F_1}{A}$$

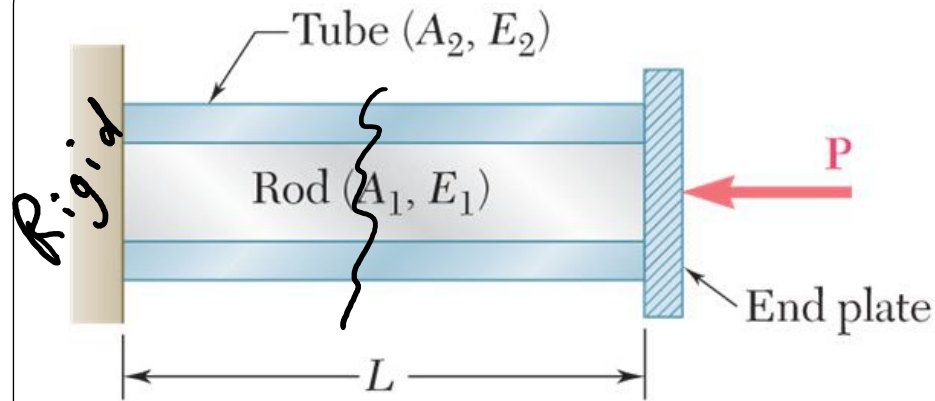
$$\sigma_2 = \frac{F_2}{A}$$

$$\delta_D = \frac{2 \cdot F_2 \cdot b}{EA}$$

$$= \frac{2b}{EA} \left( \frac{75W}{4\sqrt{5} + 25\sqrt{2}} \right)$$

$$\delta_D = \left( \frac{75}{4\sqrt{5} + 25\sqrt{2}} \right) \frac{W \cdot b}{EA}$$





Find:

- Displacement
- Stress in the rod and tube

### 3. Geometric Compatibility

$$\epsilon_r = \epsilon_t$$

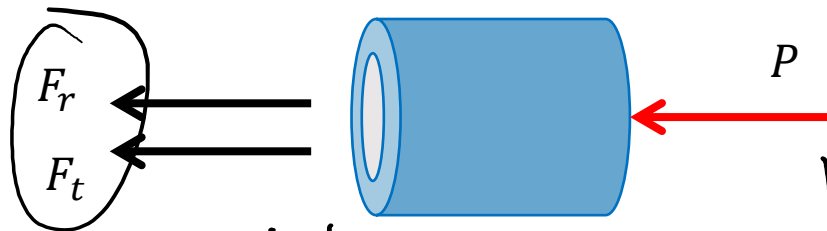
$$\frac{F_r \cdot L}{E_1 A_1} = \frac{F_t \cdot L}{E_2 A_2}$$

$$F_r = F_t \cdot \left( \frac{E_1 A_1}{E_2 A_2} \right)$$

$$F_t \cdot \left( \frac{E_1 A_1}{E_2 A_2} \right) + F_t \cdot \left( \frac{E_2 A_2}{E_2 A_2} \right) = -P$$

$$F_t \cdot \left( \frac{E_1 A_1 + E_2 A_2}{E_2 A_2} \right) = -P$$

$$F_t = \frac{-P \cdot E_2 A_2}{E_1 A_1 + E_2 A_2}$$



1. Equilibrium

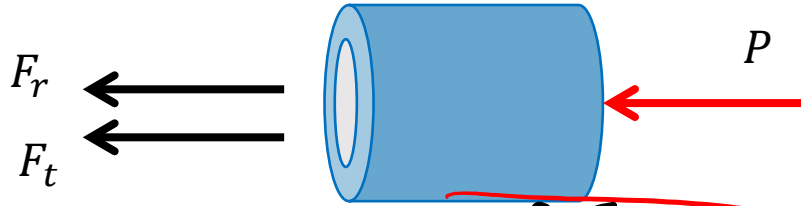
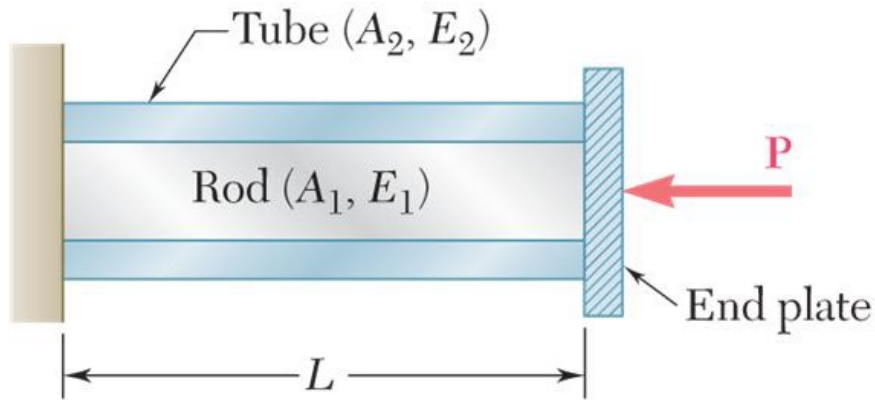
$$-F_r - F_t - P = 0$$

$$F_r + F_t = -P$$

2. Force - Elongation

$$\epsilon_r = \frac{F_r \cdot L}{E_1 A_1} \quad \left| \quad \epsilon_t = \frac{F_t \cdot L}{E_2 A_2} \right.$$





$$\sigma_r = \frac{F_r}{A_1} = \frac{-P E_1}{(EA)_1 + (EA)_2}$$

$$\sigma_t = \frac{-P \cdot E_2}{(EA)_1 + (EA)_2}$$

Find:

- Displacement
- Stress in the rod and tube ✓

$$F_r + F_t = -P$$

$$F_r + \left( \frac{-P E_2 A_2}{E_1 A_1 + E_2 A_2} \right) = -P \cdot \left( \frac{E_1 A_1 + E_2 A_2}{E_1 A_1 + E_2 A_2} \right)$$

$$F_r = \frac{P}{E_1 A_1 + E_2 A_2} \cdot [E_2 A_2 - E_1 A_1 - E_2 A_2]$$

$$F_r = \frac{-P \cdot E_1 A_1}{E_1 A_1 + E_2 A_2}$$

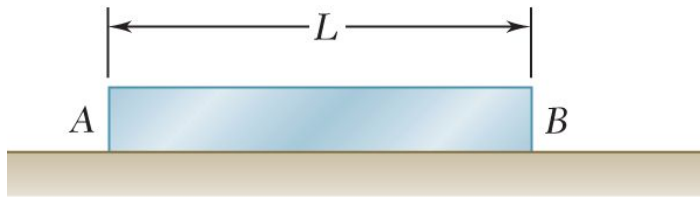
$$e = e_t = e_r = \frac{F_t \cdot L}{E_2 A_2}$$

$$e = \frac{-P \cdot L}{E_1 A_1 + E_2 A_2}$$

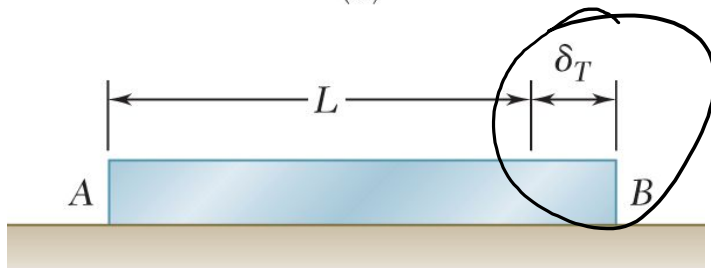
Displaces to the left



# Problems involving temperature changes



(a)



(b)

- Rod rests freely on a smooth horizontal surface
  - Temperature of the rod is raised by  $\Delta T$
  - Rod elongates by an amount
- No friction  
No mech. force applied!

$$\delta_T = \alpha L \Delta T$$

$[\alpha] = \frac{1}{\text{temp.}}$

- $\alpha$ : coefficient of thermal expansion ( $/^\circ\text{C}$ )
- This deformation is associated to a thermal strain

$$\epsilon_T = \alpha \Delta T \quad [\alpha \cdot \Delta T] = \frac{\text{temp.}}{\text{temp.}}$$

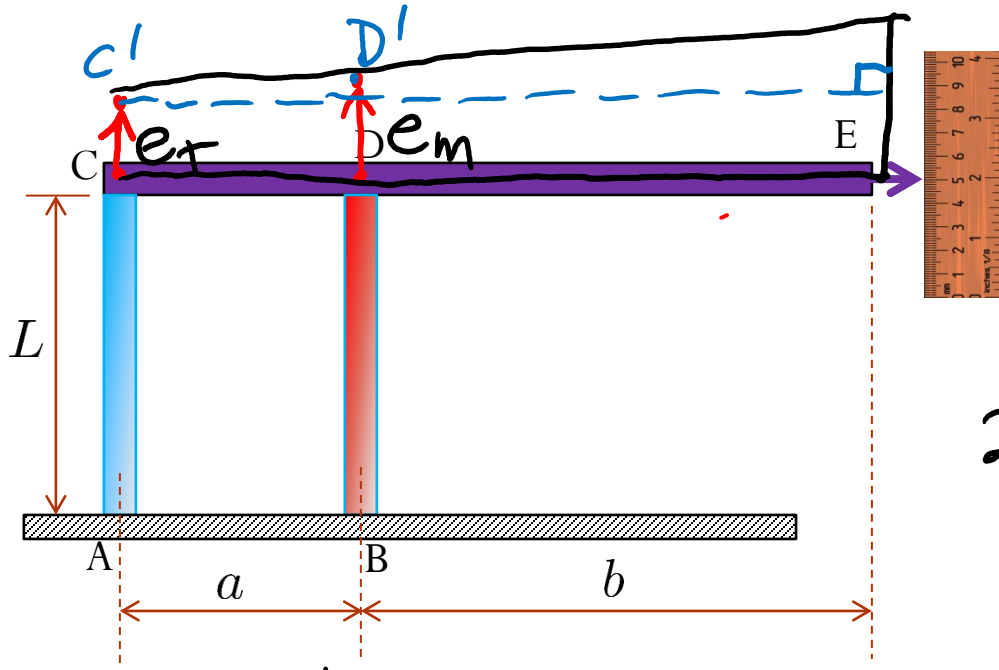
NOTE: NO STRESS is associated with the thermal strain



**Verrazano-Narrows Bridge:** Because of thermal expansion of the steel cables, the bridge roadway is 12 feet (3.66 m) lower in summer than in winter



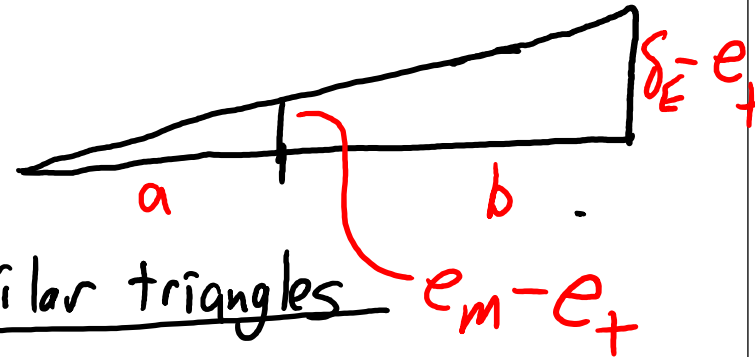
The device is used to measure a change in temperature. Rod AC and BD are made of Tungsten and Magnesium respectively. At a given temperature  $T_0$ , the rigid bar CDE is in the horizontal position. Determine an expression for the temperature  $T$  as a function of the vertical displacement of point E,  $\delta_E$ .



- Rod AC: Tungsten  $\alpha_t$
- Rod BD: Magnesium  $\alpha_m$

$$\Delta T = T - T_0 \quad \alpha_m > \alpha_t$$

2.



similar triangles  $e_m - e_t$

$$\frac{e_m - e_t}{a} = \frac{\delta_E - e_t}{a + b}$$

$$\delta_E = (e_m - e_t) \cdot \frac{a + b}{a} + e_t$$

1. Elongation

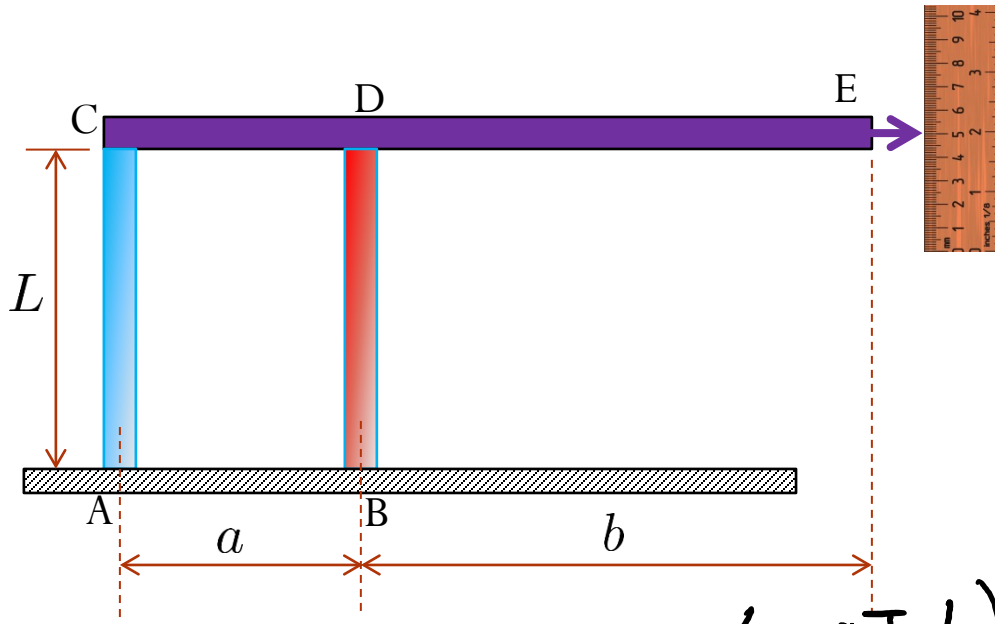
$$\text{Tungsten: } e_t = \alpha_t \cdot L \cdot \Delta T$$

$$\text{Magnesium: } e_m = \alpha_m \cdot L \cdot \Delta T$$





The device is used to measure a change in temperature. Rod AC and BD are made of Tungsten and Magnesium respectively. At a given temperature  $T_0$ , the rigid bar CDE is in the horizontal position. Determine an expression for the temperature  $T$  as a function of the vertical displacement of point E,  $\delta_E$ .



- Rod AC: Tungsten  $\alpha_t$
- Rod BD: Magnesium  $\alpha_m$

$$\alpha_m > \alpha_t$$

$$e_m = \alpha_m \Delta T \cdot L$$

$$e_t = \alpha_t \Delta T \cdot L$$

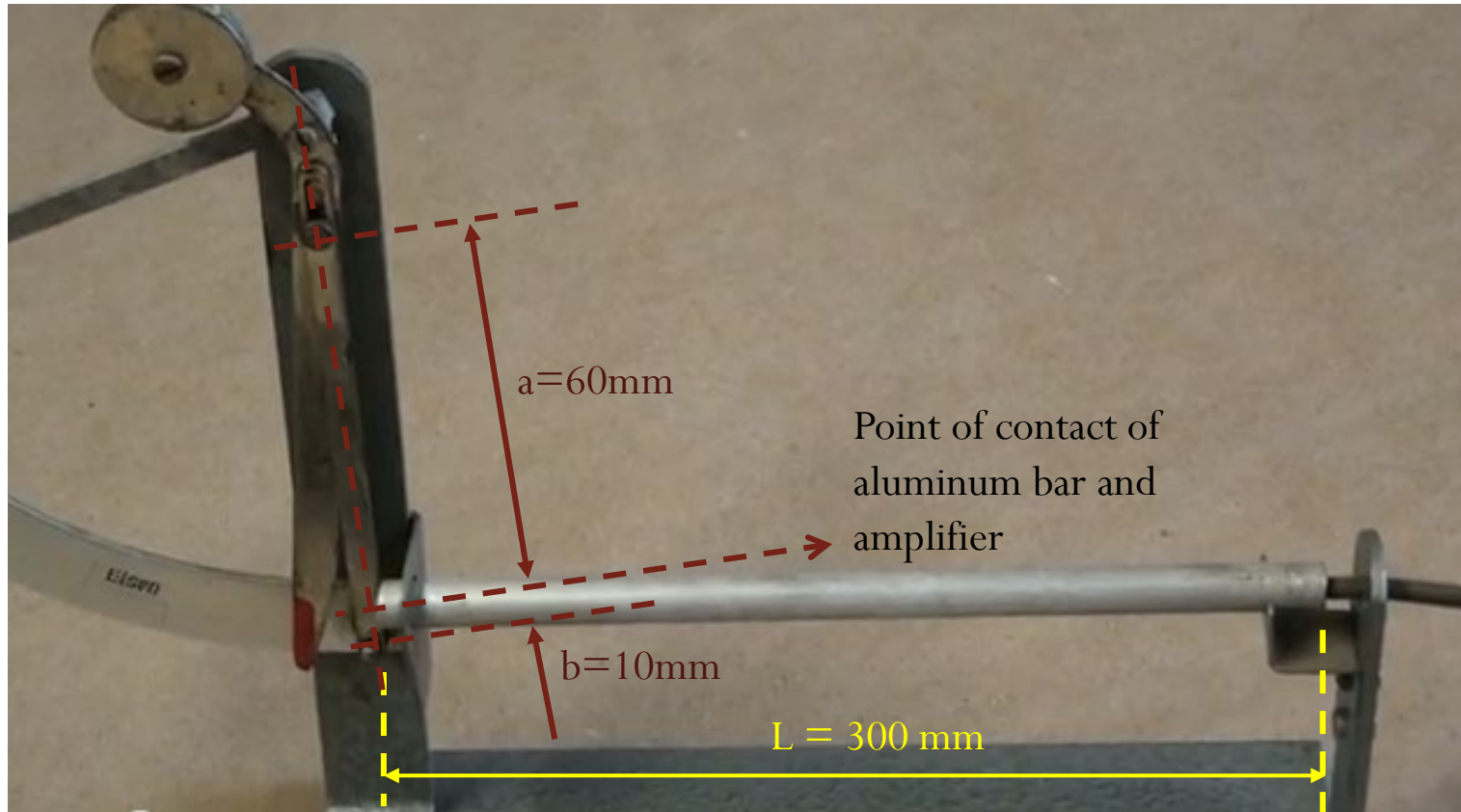
$$\begin{aligned} \delta_E &= (\alpha_m \cdot \Delta T \cdot L - \alpha_t \cdot \Delta T \cdot L) \left( \frac{a+b}{a} \right) + \alpha_t \cdot \Delta T \cdot L \\ &= \Delta T \cdot L \left[ (\alpha_m - \alpha_t) \cdot \left( \frac{a+b}{a} \right) + \alpha_t \right] \end{aligned}$$

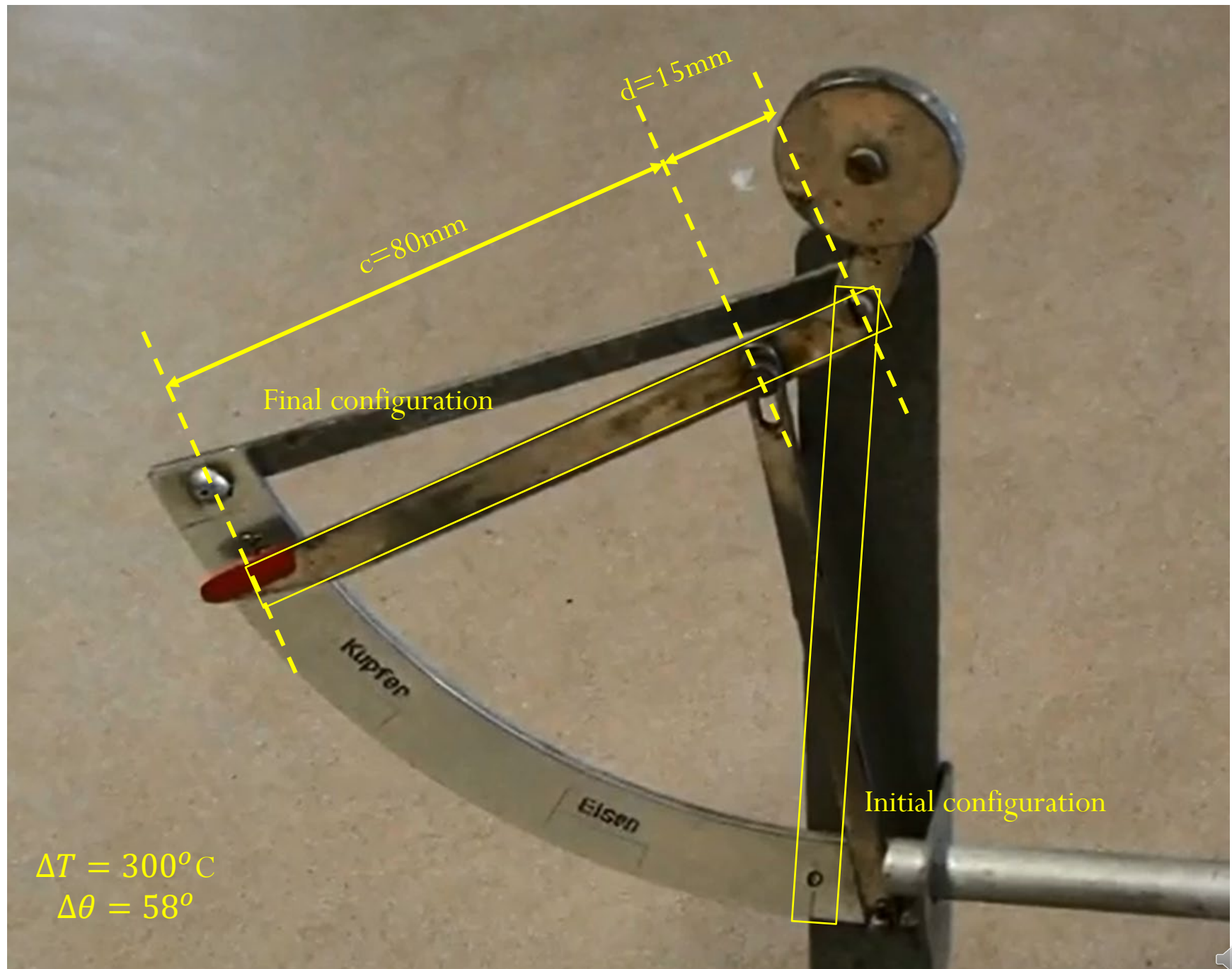
$$T = T_0 + \frac{\delta_E}{L \cdot \left[ \alpha_t + (\alpha_m - \alpha_t) \left( \frac{a+b}{a} \right) \right]}$$



# Measuring the coefficient of thermal expansion

<http://www.youtube.com/watch?v=TDnLbjd429M>





$c=80\text{mm}$

$d=15\text{mm}$

Final configuration

Initial configuration

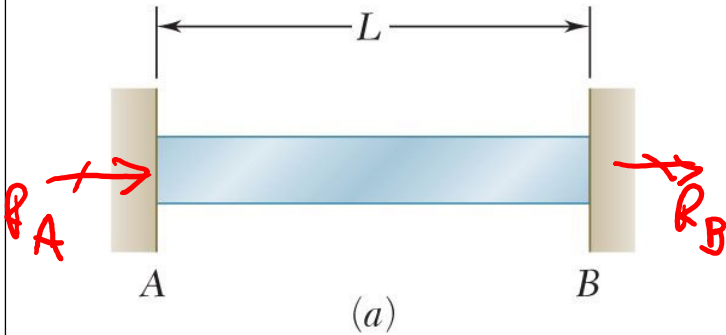
$\Delta T = 300^\circ\text{C}$   
 $\Delta \theta = 58^\circ$

Kupfer

Eisen



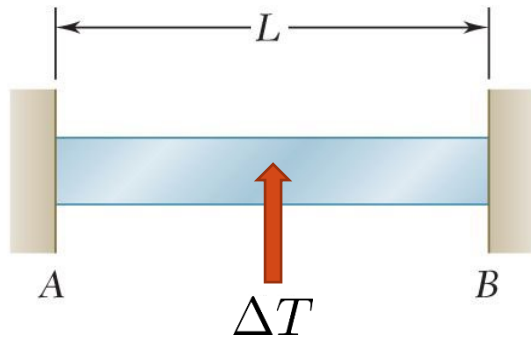
# Statically indeterminate problems



- Initially, rod of length  $L$  is placed between two supports at a distance  $L$  from each other
- No internal forces  $\longrightarrow$  no stress or strain
- Equilibrium:  $R_A = -R_B = 0$



$$F = -R_A \quad \text{Statically indeterminate problem!}$$



- After raising the temperature, the total elongation of the rod is still zero!
- The total elongation is given by

$$\delta = \underbrace{\frac{FL}{EA}}_{\text{Mechanical}} + \underbrace{\alpha L \Delta T}_{\text{thermal}} = 0$$

$$\Rightarrow F = -\alpha E A \Delta T$$

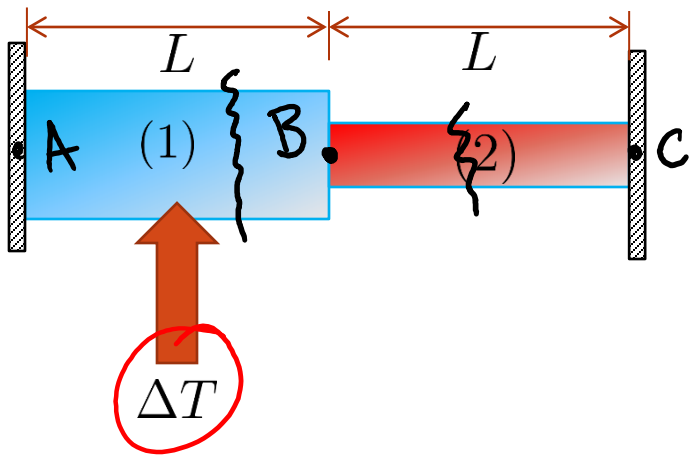
Rod is under compression



- The stress in the rod due to change in temperature is given by

$$\sigma = -\alpha E \Delta T$$

} only happens (nonzero therm  $\sigma$ ) if motion constrained



$E_1 = E_2 = E$  Assume perfect insulation at point B.  
 $\alpha_1 = \alpha_2 = \alpha$   
 $A_1, A_2$   
 Find  $\sigma_1$  &  $\sigma_2$   
 Find  $\delta_B$ .

1. Equilibrium

$\leftarrow F_1 \quad \rightarrow F_2$   
 $\sum F_x = 0 \Rightarrow F_1 = F_2 = F$  (unknown)

2. Force - elongation

$$\delta_1 = \frac{F_1 L}{E A_1} + \alpha \cdot \Delta T \cdot L$$

$$\delta_2 = \frac{F_2 L}{E A_2}$$

3. Geom. Compatibility

$$\delta_1 + \delta_2 = 0 \Rightarrow \delta_1 = -\delta_2$$

$$\frac{F}{E A_1} + \alpha \Delta T = -\frac{F}{E A_2}$$

$$\frac{F}{E} \left( \frac{1}{A_1} + \frac{1}{A_2} \right) = -\alpha \cdot \Delta T$$

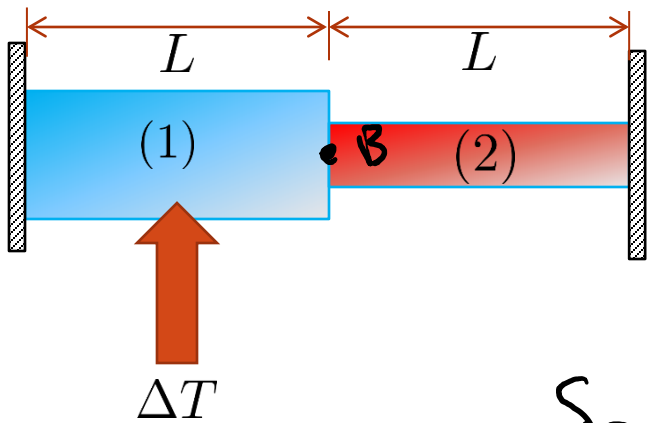
$$\Rightarrow F \left( \frac{A_1 + A_2}{A_1 A_2 E} \right) = -\alpha \Delta T$$

$$F = -\frac{\alpha \cdot \Delta T \cdot A_1 A_2 E}{A_1 + A_2}$$

$$\sigma_1 = \frac{F}{A_1} = \dots$$

$$\sigma_2 = \frac{F}{A_2} = \dots$$

$$\delta_B = \delta_1$$



$$E_1 = E_2 = E$$

$$\alpha_1 = \alpha_2 = \alpha$$

$$A_1, A_2$$

$$\delta_B = \delta_1 = \frac{F \cdot L}{E A_1} + \alpha \cdot \Delta T \cdot L$$

$$F = \frac{-\alpha \cdot \Delta T \cdot A_1 \cdot A_2 \cdot E}{A_1 + A_2}$$

$$\delta_B = \frac{-\alpha \cdot \Delta T \cdot A_2 \cdot L}{A_1 + A_2} + \alpha \cdot \Delta T \cdot L$$

$$= \alpha \cdot \Delta T \cdot L \cdot \left( 1 - \frac{A_2}{A_1 + A_2} \right)$$

$$1 = \frac{A_1 + A_2}{A_1 + A_2}$$

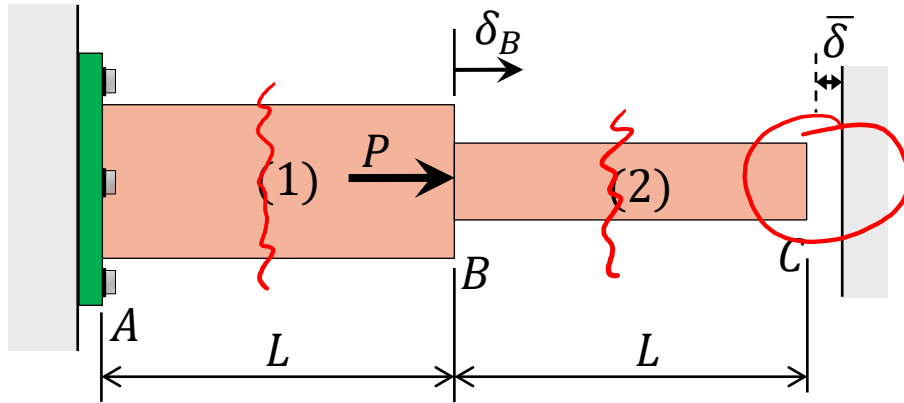
$$= \alpha \cdot \Delta T \cdot L \cdot \left( \frac{A_1 + \cancel{A_2} - \cancel{A_2}}{A_1 + A_2} \right)$$

$$\delta_B = \frac{\alpha \cdot \Delta T \cdot L \cdot A_1}{A_1 + A_2}$$

$B$  displaces to the right

# Misfit problems

$$F_1 = k_1 \cdot e_1 \quad F_2 = k_2 \cdot e_2$$



Rods (1) and (2) have stiffnesses  $k_1$  and  $k_2$ , respectively. At end A, the rod is connected to a rigid wall, but at end C there is a small gap  $\bar{\delta}$  between the original position of end C and the rigid wall. A force  $P$  is applied at point B as shown.

- What is the minimum required force  $P_{\min}$  required to close the gap between end C and the wall at the right?
- Derive expressions for the stress in each bar if the applied force is  $P > P_{\min}$ . Which bar(s) are in tension? Which are in compression?

$$F_2 \leftarrow \boxed{2} \rightarrow$$

$$F_2 = 0$$

$$F_1 \leftarrow \boxed{1} \rightarrow$$

$$F_1 = P$$

To close the gap

$$e_2 = 0, \quad e_1 = \bar{\delta}$$

$$F_1 = k_1 \cdot e_1$$

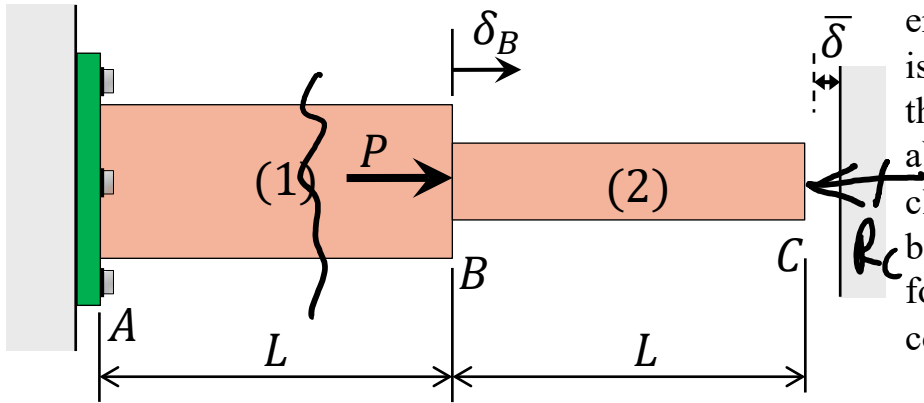
$$= k_1 \cdot \bar{\delta}$$

$$\Rightarrow P = k_1 \cdot \bar{\delta}$$

$$P_{\min} = k_1 \cdot \bar{\delta}$$

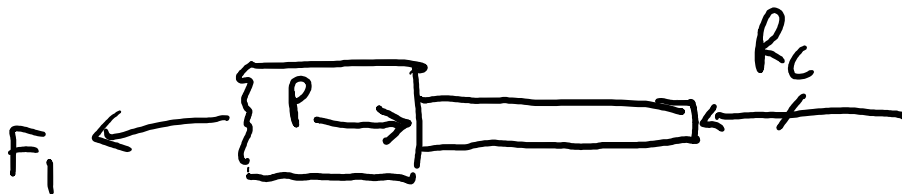


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$$-F_1 + P - R_C = 0$$



$$-F_2 - R_C = 0$$

$$-F_2 = +R_C$$

$$-F_1 + P + F_2 = 0 \Rightarrow F_1 - F_2 = P \quad (1)$$

$$e_1 = \frac{F_1}{k_1} \quad e_2 = \frac{F_2}{k_2}$$

Geom. Compat.

$$e_1 + e_2 = \bar{\delta}$$

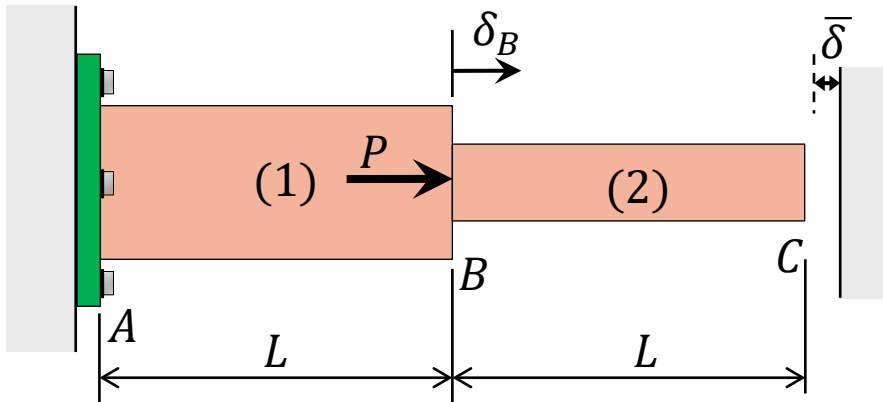
$$\frac{F_1}{k_1} + \frac{F_2}{k_2} = \bar{\delta}$$

$$\frac{P + F_2}{k_1} + \frac{F_2}{k_2} = \bar{\delta}$$

$$\Rightarrow (P + F_2)k_2 + F_2 k_1 = \bar{\delta} k_1 k_2$$



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$$(P + F_2)k_2 + F_2 \cdot k_1 = \bar{\delta} \cdot k_1 \cdot k_2$$

$$F_2 \cdot (k_1 + k_2) + P \cdot k_2 = \bar{\delta} \cdot k_1 \cdot k_2$$

$$F_2 = \frac{(\bar{\delta} \cdot k_1 - P) \cdot k_2}{k_1 + k_2}$$

$$\sigma_2 = \frac{F_2}{A_2} = \frac{(\bar{\delta} k_1 - P) \cdot k_2}{(k_1 + k_2) \cdot A_2}$$

see that  $\bar{\delta} \cdot k_1 = P_{\min} \Rightarrow (P_{\min} - P) < 0$

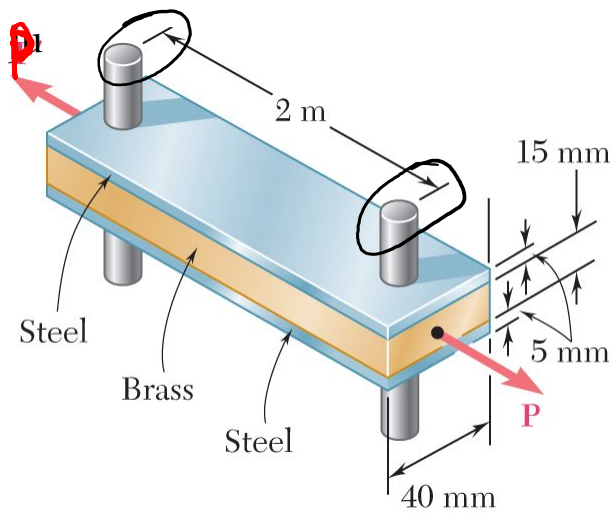
$\Rightarrow$  bar 2 is in compression!

$$F_1 = P + F_2 = P \cdot \left( \frac{k_1 + k_2}{k_1 + k_2} \right) + \frac{(\bar{\delta} k_1 - P) k_2}{k_1 + k_2}$$

$$\sigma_1 = \frac{(P + \bar{\delta} \cdot k_2) k_1}{(k_1 + k_2) A_1}$$

bar 1 is in tension!





Two steel bars ( $E_s = 200 \text{ GPa}$  and  $\alpha_s = 11.7 \times 10^{-6}/^\circ\text{C}$ ) are used to reinforce a brass bar ( $E_b = 105 \text{ GPa}$ ,  $\alpha_b = 20.9 \times 10^{-6}/^\circ\text{C}$ ) that is subjected to a load  $P = 25 \text{ kN}$ . When the steel bars were fabricated, the distance between the centers of the holes that were to fit on the pins was made  $0.5 \text{ mm}$  smaller than the  $2 \text{ m}$  needed. The steel bars were then placed in an oven to increase their length so that they would just fit on the pins. Following fabrication, the temperature in the steel bars dropped back to room temperature. Determine (a) the increase in temperature that was required to fit the steel bars on the pins, (b) the stress in the brass bar after the load is applied to it.

a) steel bars only

$$L = 2 \text{ m (desired length)}$$

$$L_{fab} = L - \delta_m = 2 \text{ m} - 0.0005 \text{ m} = 1.9995 \text{ m}$$

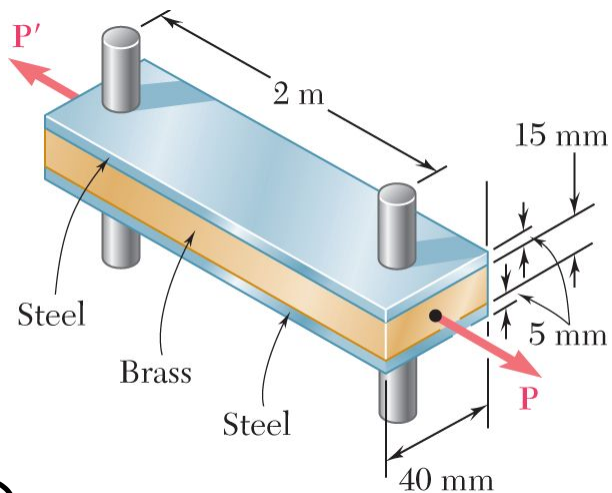
$\delta_m$   
misfit

$$\text{want } \delta_T = \alpha_s \cdot \Delta T \cdot L_{fab} = \delta_m \Rightarrow \Delta T = \frac{\delta_m}{\alpha_s \cdot L_{fab}}$$

$$\Delta T = \frac{0.0005 \cancel{\text{m}}}{(11.7 \times 10^{-6} \cancel{\%}) (1.9995 \cancel{\text{m}})} = 21.4^\circ\text{C}$$

b) After steel is pinned to the brass:

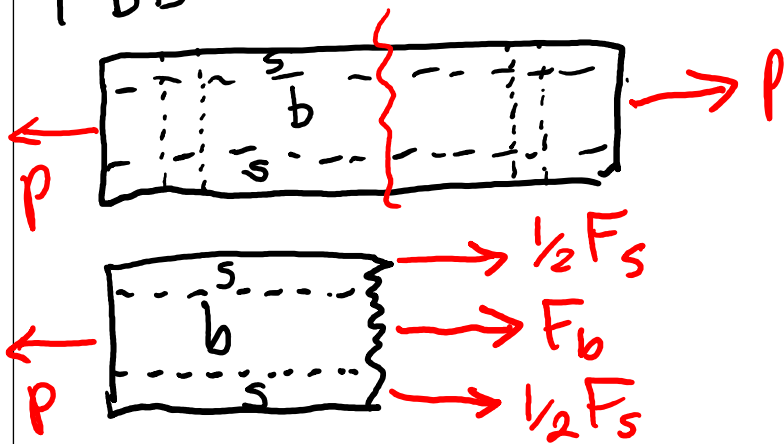
1) steel cools to init. temp. (2) steel & brass achieve a new length  $\neq L (= 2 \text{ m})$  (3) Apply  $P$  to the system



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Determine (a) the increase in temperature that was required to fit the steel bars on the pins, (b) the stress in the brass bar after the load is applied to it.

FBD



Equil.  $\sum F_x = 0$

$$-P + F_b + F_s = 0$$

$$F_b + F_s = P$$

compatibility

$$\delta_s = \delta_b$$

force-elongation

$$\delta_s = -\delta_M + \frac{F_s \cdot L}{E_s A_s}$$

return to original length upon cooling

$$\delta_b = \frac{F_b \cdot L}{E_b A_b}$$

change in length due to internal forces

} no thermal effects

$$\frac{F_s L}{E_s A_s} - \delta_M = \frac{F_b L}{E_b A_b}$$

$$\sigma_b = 3.67 \text{ MPa}$$

